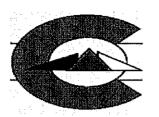
GARFIELD COUNTY BUILDING PERMIT APPLICATION

108 8th Street, Suite 401, Glenwood Springs, Co 81601 Phone: 970-945-8212 / Fax: 970-384-3470 / Inspection Line: 970-384-5003

www.garfield-county.com

1	Parcel No: (this information is available at the as	•			
2	Job Address: (if an address has not been assigned		treet Name & City) or and	legal description	
	45705 Highway	6 and 2	4 Glener	god Jorings	(08/60)
3	Lot No: 69 BI	ock No: 18	Indaud o	abd./ Exemption	ex Fistatow
4	Owner: (property owner)	Mailing Address:	Ny 6 6 24	Ph: 9 70 947 52	Alt Ph;
5	Contractor:	Mailing Address:	Julian 92.	Ph: 970 874 552	Alt Ph:
6	Architect / Engineer:	Mailing Address:	0 1416	Ph:	Alt Ph:
7	Sq. Ft. of Building:	Sq. Ft. or Acres of Lot	Penace	Height: 7	No. of Floors:
8	Use of Building:	wast three	~ a a d G		o bulling
9	Describe Work:	uesi mous	LOW PA	TH	1 20/10 2/1
10	Class of Work:	was a real	las Feet	I Charging	TO 21.5/12 P; +01.
11	□ New	Alteration	Keroot.	□ Addition	
11	Garage:	> Detached	Septic:	□ ISDS	□ Community
12	Driveway Permit:		Owners valuation of	f Work: \$	
Authorit	y. This application for a Building Permit must be signed by		FICE cribed above, or an authorize	zed agent. If the signature helo	w is not that of the Owner, a senarate
	letter of authority, signed by the Owner, must be provicess. A Building Permit cannot be issued without proof of	led with this Application.			•
	rmits. Multiple separate permits may be required: (1) Sta	te Electrical Permit, (2) County			
Void Per	County Highway/ Road Access or a State Wastew mit. A Building Permit becomes null and void if the work		rithin 180 days of the date o	f issuance and if work is susper	nded or abandoned for a period of 180
	days after commencement.	CERTIF	ICATION		
	certify that I have read this Application and that the inform fications and other data submitted by me or on my behalf (the Building Department accep	ts the Application, along with the plans
Assuming	g completeness of the submittals and approval of this Appl			to me, as Owner, to construct	the structure(s) and facilities detailed on
In consid	ittals reviewed by the Building Department. eration of the issuance of the Building Permit, I agree that				
	DS regulations and applicable land use regulations (County ion or use of the structure(s) and facility(ies), described ab				on notice from the County, if the location,
I hereby	grant permission to the Building Department to enter the pr Official from: (1) requiring the correction of errors in the s	operty, described above, to insp	ect the work. I further ackn	owledge that the issuance of the	
of County	y Regulation(s) or any other applicable law.	•			• • • • • • • • • • • • • • • • • • • •
or discrep	of this Application, including submittals, and inspections of pancies. As the Owner, I acknowledge that responsibility f				
	rmy architect designer, engineer and/or brilder. BY ACKNOWLEDGE THAD HAVE READ AND UND	ERSTAND THE NOTICE & C	ERTIFICATION ABOVE	2:	•
	101				
tre	paro it tang	TO A FOX			
OWN	ERS SIGNATURE	DATE			
		STAFF US	E ONI V		
		SIAFF OS	EUNLI		
Special	Conditions:				
Adjust	ed Valuation: Plan Check Fee:	Permit Fee:	00 M	anu home Fee:	Misc Fees:
lio.	1000.00	100.			
ISDS I		Fees Paid: 00	Balance Due:	BP No & Issue Date	: ISDS No & Issued Date:
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Setbac	ks: OCC G	roup:	Const Type		Zoning:
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BLDG	DEPT:		PLNG DEP	<u></u> <u></u>	
	MANINAM	DOUM	}		
APPRO	DYAL DATE	14/4/14	APPROVA	т	DATE
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FLAS 410000 1047 1047



Garfield County

Building & Planning Department

108 8th Street, Suite #401 Glenwood Springs, Co. 81601 Office:970-945-8212 Fax: 970-384-3470 Inspection Line: 970-384-5003

Building Permit No.

11155

Parcel No:			2123-36	1-22-001					
Locality:	Ca	nyon Cre	ek Estates 2nd	Amended, Block 18, Lot 69					
Job Address:			45705 Hwy 6	3 & 24, GWS	• • • •				
Use of Building:		re-roof							
		Pi							
Owner :			Panter,	Richard					
Contractor:			Foxworth	G@lbraith					
Fees:	Plan Check:			Septic:					
	Bldg Permit:	\$	100.00	Other Fees:					
	Total Fees:	\$	100.00						
		-							

RECEIVED

Richard Sack Rantry 45705 canyon Creeke Chemissod Springs Co.

1.) No Internet service growidor (using illegal router) F.C.C.,

For legal action.

2) Commercial groporty vented as residential rental groporty

(vithout shower or cooketop stove or Fental Occupancy

legal state or county standard.

3.) Illegal remodel shower without foundation.

4.) Used widdle name for recients and identification and in conversations, "Sack" instead of First name Richard Panty

S.) Remodled roof with Framed truss system without framing inspection and tried to get a re-roof inspection.

6) Irs problems with Girl Briend Para and Financial investment

2) Illegal vental in a Commercial Structure that is not within the Unitern Occupancy Standards

Ser a re-roof permit

Mark Collins Ro. Box 1594 Colenwood Springs Ca 81602 214-2847

Richard J. Panter 45705 Hwy 6 and 24 Glenwood Springs, CO 81601

To: Garfield County Building Permits Permit No. 11155

Enclosed:

2 copies of Structural Engineer Letter 2 copies of Truss changes Check for \$50.

I will be around my home Tuesday 9th of Feb after 11 A.M. for the rest of the day. If you need a better time my home phone is 947-5203.

Jak Jailes



t. boyle engineering, inc.

1650 e. vail valley driv falfridge unit c-2 vail, colorado 81657 (970) 476-2170 fax (970) 476-4383

May 22, 2009

Garfield County Building Department Glenwood Springs, Colorado

Subject: Panter Garage/Apartment Building Remodel 45705 U.S. Hwy. 6 Garfield County, Colorado 81601 Permit # 11155

Dear Sir/Madam:

This letter is to confirm that I have reviewed the roof modification to the Garage/Apartment building located on the Panter Property at the above noted address.

I have determined that the existing structure will be capable of supporting the new manufactured truss roof system, as well as the local 40 pound per square foot snow load. Furthermore, I have reviewed the double LVL beam and post system used to support the roof overhang at the entry, and found that these new elements will also support the code required live and dead loads.

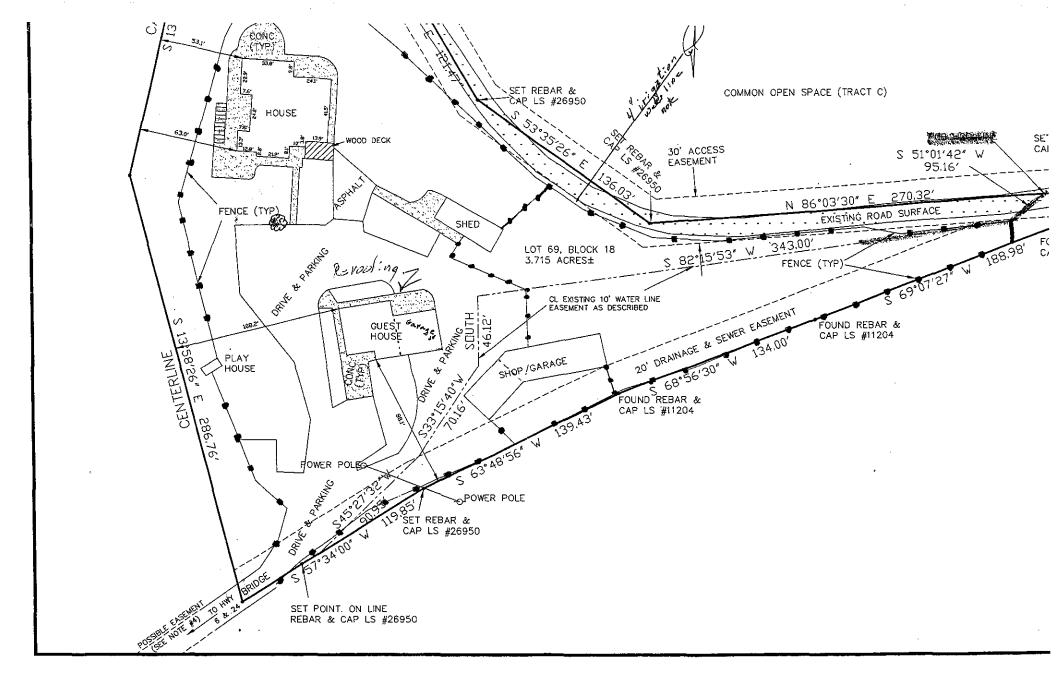
Please feel free to give me a call if you have any questions or comments on this matter.

Sincerely yours,

T. BOYLE ENGINEERING Timothy M. Boyle, P.E. C.

President

TMB/dn





MiTek Industries, Inc.

7777 Greenback Lane Suite 109 Citrus Heights, CA, 95610 Telephone 916/676-1900 Fax 916/676-1909

Re: J0809050 Jack

The truss drawing(s) referenced below have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Foxworth Galbraith (Delta).

Pages or sheets covered by this seal: R29638922 thru R29638923

My license renewal date for the state of Colorado is June 30, 2010.

GOLDWARD WERE

BUZZ WOOD FOX 8

225 SOX 4/2 A

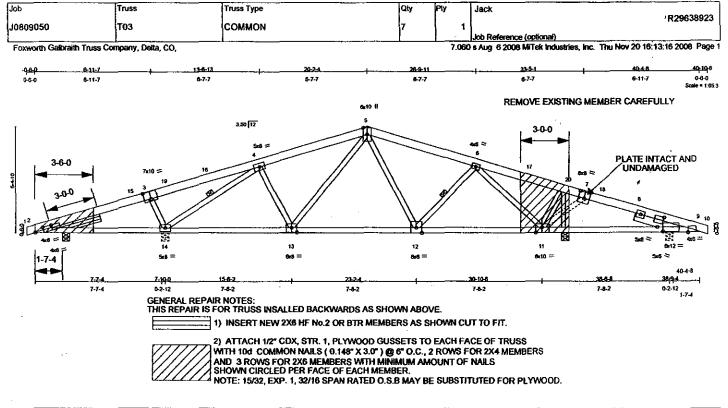
Labor 625

Muter 110

November 24,2008

Tingey, Palmer

The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-2002 Chapter 2.



.OADING (psf) FCLL 40.0 Roof Snow=40.0) FCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING 2-0-0 Plates increase 1.15 Lumber increase 1.15 Rep Stress incr YES Code IRC2006/TPI2002	CSI TC 0.46 BC 0.95 WB 0.96 (Matrix)	DEFL in (loc) Vert(LL) -0.31 11-12 Vert(TL) -0.55 11-12 Horz(TL) 0.18 9	>999 >711	L/d 360 240 n/a	PLATES GRIP MT20 169/123 Weight: 173 ib
----------------------------------------------------------------------	--------------------------------------------------------------------------------------------------	--------------------------------------------------	-------------------------------------------------------------------------	--------------	--------------------------	-----------------------------------------------

TOP CHORD

BOT CHORD

WEBS

. 30

TOP CHORD 2 X 6 SPF 2100F 1.8E BOT CHORD 2 X 4 SPF 1650F 1.5E

2 X 4 WW Stud "Except" WEBS 4-14,5-13; 2 X 4 SPF 1650F 1.5E

WEDGE

Left: 2 X 4 SYP No.3

Right 2 X 6 SPF 2100F 1.8E 3-0-8 SLIDER

REACTIONS (lb/size) 14=3054/0-5-8, 9=1886/0-5-8

Max Horz 14=85(LC 6)

Max Uplift14=-726(LC 5), 9=-367(LC 6)

Max Grav 14=3058(LC 2), 9=2056(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-15=547/1221, 3-15=533/1351, 3-19=566/1475, 16-19=563/1490, 4-16=551/1683,

4-5=-1972/376, 5-6=-3198/540, 6-17=-4637/757, 17-20=-4656/748, 7-20=-4720/743, 7-18=-5106/878, 8-18=-5145/871, 8-9=-5262/888

BOT CHORD 2-14--1167/551, 13-14--115/1551, 12-13--208/2295, 11-12--516/3930, 9-11--779/4940 WEBS

3-14=-834/306, 4-14=-3653/723, 4-13=-75/757, 5-13=-681/194, 5-12=-201/1428, 6-12=-1500/328, 6-11=-106/787, 7-11=-627/243

NOTES

1) Wind: ASCE 7-05; 90mph; TCDL=4.5psf; BCDL=4.5psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33

2) TCLL: ASCE 7-05; Pf=40.0 psf (flat roof snow); Category II; Exp C; Fully Exp.; Ct= 1

Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 20.0 psf or 2.00 times flat roof load of 40.0 psf on overhangs non-concurred with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=ib) 14=726,

7) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

ONAL

Structural wood sheathing directly applied or 4-2-0 oc purims.

4-14, 6-12

installed during truss erection, in accordance with Stabilizer

MiTek recommends that Stabilizers and required cross bracing be

Rigid ceiling directly applied or 2-2-0 oc bracing.

1 Row at midpt

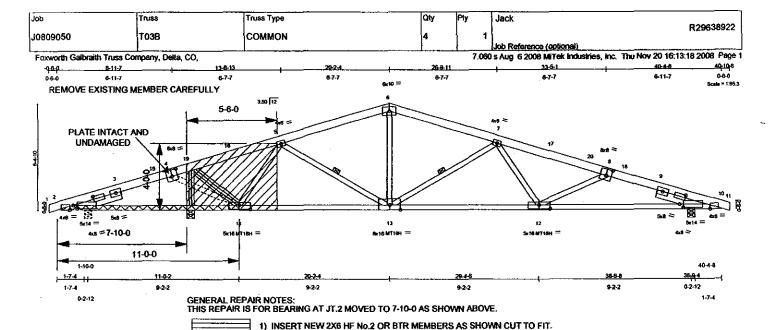
Installation guide

November 24,2008

🛕 WARNING - Verify design parameters and RRAD NOTES ON THIS AND INCLUDED MITEK REPERENCE PAGE MII-7473 BEFORE USE. Design void for use only with Mifet connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer—not hus designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the calcing designer and incorporate in the responsibility of the building designer and incorporate in the responsibility of the calcing designer for general guidance regarding tobrication, quality control, storage, detivery, exection and bracing, consult.
AMSURTIO Quality Control, storage, detivery, exection and bracing, consult.
AMSURTIO Quality Control, storage, detivery, exection and bracing, consult.
AMSURTIO Quality Control. Storage is a stability of the foliation of the control storage.
Safety information: available from truss Plate Institute, 583 D'Onofrio Drive, Mackson, WI 53719.



Citrus Heights, CA, 95610



2) ATTACH 1/2" CDX, STR. 1, PLYWOOD GUSSETS TO EACH FACE OF TRUSS WITH 10 D. COMMON NAILS (0.148" X 3.0") @ 6" O.C., 2 ROWS FOR 2X4 MEMBERS AND 3 ROWS FOR 2X6 MEMBERS WITH MINIMUM AMOUNT OF NAILS SHOWN CIRCLED PER FACE OF EACH MEMBER

NOTE: 15/32, EXP. 1, 32/16 SPAN RATED O.S.B MAY BE SUBSTITUTED FOR PLYWOOD.

3) ATTACH 2X4 SPF 1650F 1.5E SCABS AS SHOWN TO EACH SIDE OF TRUSS WITH 16d COMMON NAILS (0.162" X 3.5") 2 ROWS @ 6" O.C. ADD 1/2" PLYWOOD SHIMS WHERE SCABS NOT OVER PLAYWOOD, USE SCAB NAILS FOR GUSSET NAILS, DO NOT DOUBLE NAIL

BRACING

WEBS

TOP CHORD

BOT CHORD

1 Row at midpt

Installation guide

Plate Offsets (X,Y): [2:0-2-9.Edge], [2:1-3-14,0-2-0], [2:0-2-13,0-2-8], [4:0-4-0,0-4-8], [8:0-4-0,0-4-8], [10:0-2-13,0-2-8], [10:1-10-3,Edge], [10:0-3-0,0-7-8], [12:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-3-0], [14:0-8-0,0-

LOADING (psf) TCLL 40.0 (Roof Snow=40.0) —TCDL 10.0 BGLL 0.0 BCDL 10.0	SPACING 2-0-0 Plates Increase 1.15 Lumber Increase 1.15 Rep Stress Incr YES Code IRC2006/TPI2002	C\$I TC 0.39 BC 0.82 WB 0.43 (Matrix)	DEFL in (loc) Vdefl L/d Vert(LL) -0.51 13-14 >942 360 Vert(TL) -0.97 12-13 >496 240 Horz(TL) 0.34 10 n/a n/a	PLATES GRIP MT20 169/123 MT18H 169/123 Weight: 176 lb
------------------------------------------------------------------------	--------------------------------------------------------------------------------------------------	---------------------------------------------------	--------------------------------------------------------------------------------------------------------------	----------------------------------------------------------------

LUMBER

SLIDER

TOP CHORD 2 X 6 SPF 2100F 1.8E BOT CHORD 2 X 4 SPF 2100F 1.8E 2 X 4 WW Stud *Except* WEBS

5-13,6-13,7-13: 2 X 4 SPF 1650F 1.5E

Left 2 X 6 SPF 2100F 1.8E 3-0-4, Right 2 X 6 SPF 2100F 1.8E 3-0-4

REACTIONS (lb/size) 2=2468/0-5-8, 10=2468/0-5-8

Max Horz 2=85(LC 5)

Max Uplift2=432(LC 5), 10=432(LC 6) Max Grav2=2471(LC 2), 10=2471(LC 3)

FORCES (b) - Max. Comp./Max. Ten. - All forces 250 (b) or less except when shown.

TOP CHORD 2-3=6574/1079, 3-15=6447/1089, 4-15=6420/1095, 4-19=5899904, 16-19=5838/908,

5-16=-5816/918, 5-6=-4326/671, 6-7=-4326/671, 7-17=-5816/918, 17-20=-5838/909, 8-20-5899/904, 8-18-6420/1096, 9-18-6447/1090, 9-10-6574/1080

2-14-1069/6189, 13-14-811/5294, 12-13-726/5294, 10-12-985/6189

BOT CHORD 4-14-658/268, 5-14-36/584, 5-13-1860/369, 6-13-196/1692, 7-13-1860/369, WERS

7-12=-37/584, 8-12=-658/269

NOTES

1) Wind: ASCE 7-05; 90mph; TCDL=4.5psf; BCDL=4.5psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33

2) TCLL: ASCE 7-05; Pf=40.0 psf (flat roof snow); Category II; Exp C; Fully Exp.; Ct= 1
3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 20.0 psf or 2.00 times flat roof load of 40.0 psf on overhangs non-conci with other live loads

5) All plates are MT20 plates unless otherwise indicated.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=tb) 2=432, 10=432

8) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

November 24,2008

🛕 WARNING - Verify dosign parameters and READ NOTES ON THIS AND INCLUDED WITEK REFERENCE PAGE MII-7473 BEFORE USE. MANNINY: VENTY EASING PARAMETERS AND INCLUDING THE DAY THE DAY THE DAY THE DAY BELLEY OF THE DAY BELLE



SS/ONAL

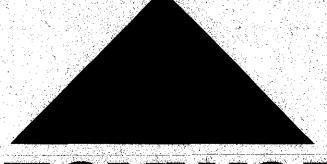
Structural wood sheathing directly applied or 3-7-13 oc purlins. Rigid ceiling directly applied or 7-6-14 oc bracing.

5-13, 7-13

installed during truss erection, in accordance with Stabilizer

MiTek recommends that Stabilizers and required cross bracing be

Citrus Heights, CA, 95610



FOXWORTH CALBRAITH

19440 Highway 92 Delta, CO 81416 970-874-5522

FAX: 970-874-1300

PACKAGE PREPARED EXCLUSIVELY FOR:

Jack.
45705 Hwy 6.
Glenwood Springs, Co.
October 7, 2008
Project #J0809050

FOXWORT GALBRAIT	H



19440 Highway 92 • Delta, CO 81416

(970) 874-5522 • (970) 874-9865 (Fax)

		DATE	10/06/08 PAGE
REQ. QUOTE DATE	11	ORDER#	J0809050
ORDER DATE	09/23/08	QUOTE#	B0809050
DELIVERY DATE	10/07/08	CUSTOMER ACCT #	355548311
DATE OF INVOICE	11	CUSTOMER PO #	
ORDERED BY		INVOICE #	
		TERMS	
SUPERINTENDENT		SALES REP	Scott Bulla
JOBSITE PHONE #		SALES AREA	Grand Junction

SOL	Walk-In	JOB NAME: Jack MODEL: Jack	TAG:	LOT # SUBDIV: JOB CATEGORY:ROOF
D FO	,	DELIVERY INSTRUCTIONS: deliver with goose neck		
SHIP FO	Walk-In , 45705 HWY 6 45705 HWY 6 Glenwood Springs, CO	SPECIAL INSTRUCTIONS:		

_				_				D1	DATE
BUILDING DEPARTMENT	OVERHAN	G INFO	HEEL HEIGHT	00-04-03	REQ. LAYOUTS	REQ. ENGINEERING	QUOTE	RE	09/16/08
RE	END CUT I	RETURN			NONE	NONE	LAYOUT	RE	09/16/08
	PLUMB		GABLE STUDS	16 IN. OC	NONE	NONE	CUTTING	RE	09/16/08

ROOF TRUSSES LOADING INFORMATION					TCLL-TCDL-BCLL-BCDL STRESS INCR. 40.0,10.0,0.0,10.0 1.15				ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)														
PROFILE	QTY	PIT	СН	TYPE	BASE	O/A	LUN	IBER	OVERHANG		OVERHANG		OVERHANG		CANTILEVER		OVERHANG CANT		STUB		ER STUB		HEIGHT
	PLY	TOP	BOT	ID	SPAN	SPAN	TOP	вот	LEFT	RIGHT	LEFT	RIGHT	LEFT	RIGHT									
	2	3.50	0.00	GABLE T01	32-08-00	32-08-00	2 X 6	2 X 4	00-06-00	00-06-00	03-00-08	03-00-08			05-04-10								
	16	3.50	0.00	COMMON T01A	32-08-00	32-08-00	2 X 6	2 X 4	00-06-00	00-06-00	03-00-08	03-00-08			05-04-10								
	7	3.50	0.00	COMMON T03	40-04-08	40-04-08	2 X 6	2 X 4	00-06-00	00-06-00	07-07-04	01-07-04			06-06-02								
	1	3.50	0.00	GABLE T03A	40-04-08	40-04-08	2 X 6	2 X 4	00-06-00	00-06-00	07-07-04	01-07-04			06-06-02								
	4	3.50	0.00	COMMON T03B	40-04-08	40-04-08	2 X 6	2 X 4	00-06-00	00-06-00	01-07-04	01-07-04			06-06-02								
	1	3.50	0.00	GABLE T03C	40-04-08	40-04-08	2 X 6	2 X 4	00-06-00	00-06-00	01-07-04	01-07-04			06-06-02								

ITEMS

QTY	ITEM TYPE	SIZE	LENGTH FT-IN-16	PART NUMBER	NOTES	
62	Hanger	One H2.5T				
84	Stabilizer	Truss Stabilizer				

THE ABOVE LISTED ITEMS HAVE BEEN RECEIVED IN GOOD CONDITION. (EXCEPTIONS NOTED)
RECEIVED BY:
DATE:

THANK YOU FOR YOUR BUSINESS.



MiTek Industries, Inc.

7777 Greenback Lane Suite 109 Citrus Heights, CA, 95610 Telephone 916/676-1900 Fax 916/676-1909

Re: J0809050

Jack

The truss drawing(s) referenced below have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Foxworth Galbraith (Delta).

Pages or sheets covered by this seal: R29364542 thru R29364547

My license renewal date for the state of Colorado is June 30, 2010.



September 24,2008

Tingey, Palmer

The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-2002 Chapter 2.

Job Ply Jack Truss Qty Truss Type R29364542 J0809050 T01 GABLE Job Reference (optional) 7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Sep 24 14:01:37 2008 Page 1 Foxworth Galbraith Truss Company, Delta, CO., Dean McFate 29-4-12 6-6-6 TRUSSES IN THIS JOB ATTIC ACCESS LOAD NOT CONSIDERED 5x10 = (20 PSF 24"WX42"H) 3 50 12 6x12 == 3×16 == 5x12 = 3x4 II 29-7-6-6-6 0-2-12

Plate Offsets (X,Y): [3:0-3-12,0-5-0], [7:0-3-12,0-5-0], [11:0-3-8,0-3-0], [13:0-3-8,0-3-0], [17:0-1-8,0-0-8], [20:0-1-8,0-0-8], [23:0-1-8,0-0-8], [36:0-1-8,0-0-8], [39:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-0-8], [42:0-1-8,0-8], [42:0-1-8,0-8], [42:0-1-8,0-8], [42:0-1-8,0-8], [42:0-1-8,0-8], [

LOADING (psf) TCLL 40.0 (Roof Snow=40.0) TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING 2-0-0 Plates Increase 1.15 Lumber Increase 1.15 Rep Stress Incr NO Code IRC2006/TPI2002	CSI TC 0.34 BC 0.56 WB 0.97 (Matrix)	DEFL in (loc) l/defl L/d Vert(LL) -0.12 11-12 >999 360 Vert(TL) -0.24 12-13 >999 240 Horz(TL) 0.05 10 n/a n/a	PLATES GRIP MT20 169/123 Weight: 172 lb
-----------------------------------------------------------------------	-------------------------------------------------------------------------------------------------	--------------------------------------------------	-----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------	-----------------------------------------------

BRACING

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 6 SPF 2100F 1.8E BOT CHORD 2 X 4 SPF 1650F 1.5E WEBS 2 X 4 WW Stud *Except*

3-13,7-11: 2 X 4 SPF 1650F 1.5E

OTHERS 2 X 4 WW Stud

REACTIONS (lb/size) 14=2010/0-5-8, 10=2010/0-5-8

Max Horz 14=69(LC 5)

Max Uplift14=-443(LC 5), 10=-443(LC 6) Max Grav14=2077(LC 2), 10=2077(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/5, 2-3=-225/337, 3-49=-2745/357, 49-50=-2644/364, 4-50=-2621/371, 4-5=-2205/344, 5-6=-2205/344, 6-51=-2621/372,

51-52=-2644/364, 7-52=-2745/357, 7-8=-224/337, 8-9=0/5

BOT CHORD 2-14=-283/222, 13-14=-237/167, 12-13=-330/2538, 11-12=-260/2538, 10-11=-237/192, 8-10=-283/220 WEBS 3-14=-1933/453, 3-13=-441/2662, 4-13=-336/141, 4-12=-695/125, 5-12=-7/453, 6-12=-695/125, 6-11=-

3-14=-1933/453, 3-13=-441/2662, 4-13=-336/141, 4-12=-695/125, 5-12=-7/453, 6-12=-695/125, 6-11=-336/141, 7-11=-438/2662, 7-10=-1933/453

7-11-430/2002, 1-10-1303/-

NOTES

1) Wind: ASCE 7-05; 90mph; TCDL=4.5psf; BCDL=4.5psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33

2) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.

3) TCLL: ASCE 7-05; Pf=40.0 psf (flat roof snow); Category II; Exp C; Fully Exp.; Ct= 1

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 20.0 psf or 2.00 times flat roof load of 40.0 psf on overhangs non-concurrent with other live loads.

All plates are 2x4 MT20 unless otherwise indicated.

7) Gable studs spaced at 1-4-0 oc.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 443 lb uplift at joint 14 and 443 lb uplift at 10.

10) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

AQ130

Structural wood sheathing directly applied or 5-8-13 oc purlins.

installed during truss erection, in accordance with Stabilizer

MiTek recommends that Stabilizers and required cross bracing be

Rigid ceiling directly applied or 6-0-0 oc bracing.

Installation guide.

September 24,2008

WARNING - Verify design parameters and RRAD NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USB.

Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component.

Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to Insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fobrication, quality control, storage, delivery, erection and bracing, consult. AMSI/IT11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 6 SPF 2100F 1.8E BOT CHORD 2 X 4 SPF 1650F 1.5E WEBS 2 X 4 WW Stud *Except* 3-13,7-11: 2 X 4 SPF 1650F 1.5E

REACTIONS (lb/size) 14=2010/0-5-8, 10=2010/0-5-8 Max Horz 14=69(LC 5)

Max Uplift14=-443(LC 5), 10=-443(LC 6) Max Grav 14=2077(LC 2), 10=2077(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD

1-2=0/5, 2-3=-225/337, 3-15=-2745/357, 15-16=-2644/364, 4-16=-2621/371, 4-5=-2205/344, 5-6=-2205/344, 6-17=-2621/372,

17-18=-2644/364, 7-18=-2745/357, 7-8=-224/337, 8-9=0/5

BOT CHORD 2-14=-283/222, 13-14=-237/167, 12-13=-330/2538, 11-12=-260/2538, 10-11=-237/192, 8-10=-283/220 WEBS

3-14=-1933/453, 3-13=-441/2662, 4-13=-336/141, 4-12=-695/125, 5-12=-7/453, 6-12=-695/125, 6-11=-336/141,

7-11=-438/2662, 7-10=-1933/453

NOTES

- 1) Wind: ASCE 7-05; 90mph; TCDL=4.5psf; BCDL=4.5psf; h=25ff; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- TCLL: ASCE 7-05; Pf=40.0 psf (flat roof snow); Category II; Exp C; Fully Exp.; Ct= 1
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 20.0 psf or 2.00 times flat roof load of 40.0 psf on overhangs non-concurrent
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 443 lb uplift at joint 14 and 443 lb uplift at joint
- 7) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

SIONAL

Structural wood sheathing directly applied or 5-8-13 oc purlins.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

Rigid ceiling directly applied or 6-0-0 oc bracing.

Installation guide.

学・製作制の理解を開かります。 モングー

Alla Maria de

September 24,2008

🎪 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual bulding component. Applicability of design paramenters and proper incorporation of component is responsibility of bulding designer - not frust designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to Insulating construction is the responsibility of the serector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding individual web reports of the overall structure is the responsibility of the building designer. For general guidance regarding individual web reports of the overall structure is the responsibility of the building designer. For general guidance regarding individual control, storage, delivery, erection and bracing, consult. ANSI/TPII Quality Criteria, DSB-89 and BCSII Building Component Safety Information available from lives Plate Institute, SSB 9 Onofitio Drive, Madson, WI SS719.



7777 Greenback Lane, Suite 109 Citrus Heights, CA, 95610

Job Truss Truss Type Oty Ply Jack R29364544 J0809050 T03 COMMON 1 Job Reference (optional)
7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Sep 24 14:01:39 2008 Page 1

Foxworth Galbraith Truss Company, Delta, CO., Dean McFate

Structural wood sheathing directly applied or 4-2-0 oc purlins.

4-14, 6-12

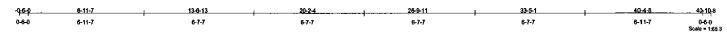
installed during truss erection, in accordance with Stabilizer

MiTek recommends that Stabilizers and required cross bracing be

Rigid ceiling directly applied or 2-2-0 oc bracing.

1 Row at midot

Installation guide.



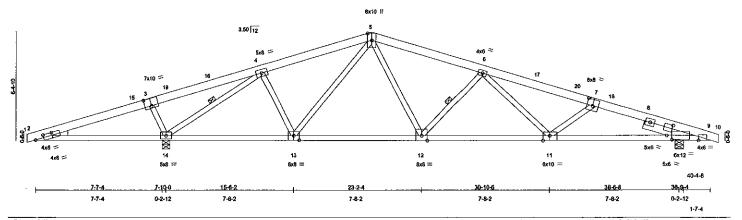


Plate Offsets (X,Y): [2:0-	-6-1,0-2-0], [2:1-0-1,0-2-0], [3:0-5-0,0-5	-4], [7:0-4-0,0-4-8], [9:0-	3-13,0-3-0], [9:1-10-3,Edge], [9):0-2-10,0-8-0], [11:0-4	I-12,Edge], [13:0-4-0,0-3-4]
LOADING (psf) TCLL 40.0 (Roof Snow=40.0) TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING 2-0-0 Plates increase 1.15 Lumber increase 1.15 Rep Stress incr YES Code iRC2006/TPI2002	CSI TC 0.46 BC 0.95 WB 0.96 (Matrix)	DEFL in (loc Vert(LL) -0.31 11-12 Vert(TL) -0.55 11-12 Horz(TL) 0.18	>999 360	PLATES GRIP MT20 169/123 Weight: 173 lb

BRACING

WEBS

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 6 SPF 2100F 1.8E BOT CHORD 2 X 4 SPF 1650F 1.5E 2 X 4 WW Stud *Except* WEBS

4-14,5-13; 2 X 4 SPF 1650F 1.5E

WEDGE Left: 2 X 4 SYP No.2

SLIDER Right 2 X 6 SPF 2100F 1.8E 3-0-8

REACTIONS (lb/size) 14=3054/0-5-8, 9=1886/0-5-8

Max Horz 14=-85(LC 6)

Max Uplift14=-726(LC 5), 9=-367(LC 6) Max Grav 14=3058(LC 2), 9=2056(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/6, 2-15=-547/1221, 3-15=-533/1351, 3-19=-566/1475, 16-19=-563/1490, 4-16=-551/1683, 4-5=-1972/376,

5-6=-3198/540, 6-17=-4637/757, 17-20=-4656/748, 7-20=-4720/743, 7-18=-5108/878, 8-18=-5145/871, 8-9=-5262/868,

BOT CHORD 2-14=-1167/551, 13-14=-115/1551, 12-13=-208/2295, 11-12=-516/3930, 9-11=-779/4940

WEBS 3-14=-834/306, 4-14=-3653/723, 4-13=-75/757, 5-13=-681/194, 5-12=-201/1428, 6-12=-1500/328, 6-11=-106/787,

NOTES

1) Wind: ASCE 7-05; 90mph; TCDL=4.5psf; BCDL=4.5psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33

2) TCLL: ASCE 7-05; Pf=40.0 psf (flat roof snow); Category II; Exp C; Fully Exp.; Ct= 1

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 20.0 psf or 2.00 times flat roof load of 40.0 psf on overhangs non-concurrent

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 726 lb uplift at joint 14 and 367 lb uplift at joint 9.

7) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

ONAL

September 24,2008

▲ WARNING - Verify design parameters and READ NOTES ON TEIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE. Design void for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for toteral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the stability during construction is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult.

ARSI/IPI AND SERSE OF THE Building Component Safety Information available from Iruss Plate Institute, 583 D'Onofilo Drive, Madison, WI 53719.



7777 Greenback Lane, Suite 109 Citrus Heights, CA, 95610

Foxworth Galbraith Truss Company, Delta, CO., Dean McFate

7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Sep 24 14:01:41 2008 Page 1

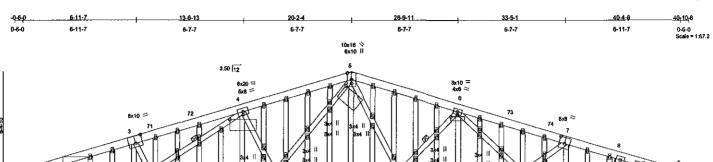
6x10 =

Structural wood sheathing directly applied or 4-2-3 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing.

4-14 6-12

installed during truss erection, in accordance with Stabilizer

MiTek recommends that Stabilizers and required cross bracing be



7-7-4 7-100 156-2 23-2-4 30-10-5 38-6-8 38-0-4 7-7-4 0-2-12 7-8-2 7-8-2 7-8-2 1-7-4 1-7-4

12

8x8 =

Plate Offsets (X,Y): [2:0-6-1,0-2-0], [2:2-6-8,0-2-10], [3:0-5-0,0-5-4], [4:0-10-0,0-3-7], [5:0-1-14,0-3-15], [6:0-5-0,0-1-0], [7:0-4-0,0-4-8], [9:0-2-14,0-8-0], [9:1-10-3,Edge], [9:0-3-13,0-2-8], [11:0-5-0,0-2-8], [11:0-5-0,0-2-8], [11:0-4-12,Edge], [13:0-4-0,0-3-4]

LOADING (psf) TCLL 40.0 (Roof Snow=40.0) TCDL 10.0 BCLL 0.0	SPACING 2-0-0 Plates Increase 1.15 Lumber Increase 1.15 Rep Stress Incr NO Code IRC2006/TPI2002	CSI TC 0.53 BC 0.80 WB 0.96 (Matrix)	DEFL in (loc) I/defl L/d Vert(LL) -0.30 11-12 >999 360 Vert(TL) -0.52 11-12 >741 240 Horz(TL) 0.17 9 n/a n/a	PLATES GRIP MT20 169/123 Weight: 246 lb
BCDL 10.0	Code 11(02000/17/2002	(IVIAUIX)	<u>.</u>	Weight 240 ib

BRACING

WEBS

TOP CHORD

BOT CHORD

1 Row at midot

Installation guide.

LUMBER

 \widehat{A}

TOP CHORD 2 X 6 SPF 2100F 1.8E BOT CHORD 2 X 4 SPF 1650F 1.5E *Except*

9-11: 2 X 4 SPF 2100F 1.8E WEBS 2 X 4 WW Stud *Except*

4-14,5-13: 2 X 4 SPF 1650F 1.5E

OTHERS 2 X 4 WW Stud

WEDGE Left: SLIDER

Right 2 X 6 SPF 2100F 1.8E 3-0-8

REACTIONS (lb/size) 14=3054/0-5-8, 9=1886/0-5-8

Max Horz 14=-85(LC 6)

Max Uplift14=-726(LC 5), 9=-367(LC 6) Max Grav14=3058(LC 2), 9=2056(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/6, 2-3=-547/1351, 3-71=-566/1475, 71-72=-563/1490, 4-72=-551/1683, 4-5=-1972/376, 5-6=-3198/540, 6-73=-4642/758,

73-74=-4660/749, 7-74=-4725/744, 7-8=-5158/880, 8-9=-5273/870, 9-10=0/6

BOT CHORD 2-14=-1167/551, 13-14=-115/1550, 12-13=-208/2295, 11-12=-515/3928, 9-11=-781/4951

WEBS 3-14=-835/306, 4-14=-3653/723, 4-13=-75/757, 5-13=-682/194, 5-12=-200/1426, 6-12=-1497/328, 6-11=-108/794,

7-11=-636/244

NOTES

1) Wind: ASCE 7-05; 90mph; TCDL=4.5psf; BCDL=4.5psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33

2) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANS/TPI 1-2002.

3) TCLL: ASCE 7-05; Pf=40.0 psf (flat roof snow); Category II; Exp C; Fully Exp.; Ct= 1

4) Unbalanced snow loads have been considered for this design.

s) this truss has been designed for greater of min roof live load of 20.0 psf or 2.00 times flat roof load of 40.0 psf on overhangs non-concurre with other live loads.

6) All plates are 2x4 MT20 unless otherwise indicated

7) Gable studs spaced at 1-4-0 oc.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 726 lb uplift at joint 14 and 367 lb uplift at joint 9.

10) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

read AQ100

September 24,2008

5x12 =

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component.

Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not fruss designer. Bracing shown is for toleral support of individual web members and ry. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult.

ANS/IPI (audity Chiefle, DSB-89 and BCSI1 Building Component Safety Information available from truss Plate Institute, SSB D'Onotifo Drive, Madison, WI 53719.



7777 Greenback Lane, Suite 109 Citrus Helghts, CA, 95610

Job	Truss	Truss Type	Qty	Ply	Jack	P20364546
J0809050	тозв	соммон	4	1	Job Reference (optional)	1723304040

Foxworth Galbraith Truss Company, Delta, CO., Dean McFate

7.060 s Aug 6 2008 MiTek Industries, Inc. Wed Sep 24 14:01:42 2008 Page 1



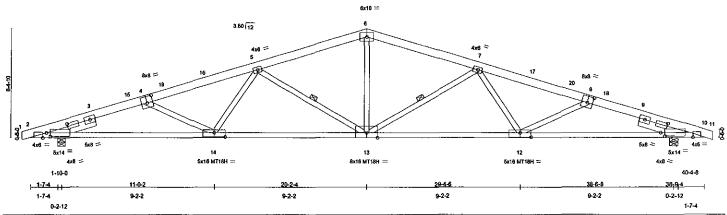


Plate Offsets (X,Y): [2:0-2-9,Edge], [2:1-3-14,0-2-0], [2:0-2-13,0-2-8], [4:0-4-0,0-4-8], [8:0-4-0,0-4-8], [10:0-2-13,0-2-8], [10:1-10-3,Edge], [10:0-3-0,0-7-8], [12:0-8-0,0-3-0], [14:0-8-0,0-3-0]

LOADING (psf) TCLL 40.0 (Roof Snow=40.0) TCDL 10.0	SPACING 2-0-0 Plates Increase 1.15 Lumber Increase 1.15	CSI TC 0.39 BC 0.82	DEFL in (loc) I/defl L/d Verl(LL) -0.51 13-14 >942 360 Verl(TL) -0.97 12-13 >496 240	PLATES GRIP MT20 169/123 MT18H 169/123
TCDL 10.0 BCLL 0.0 BCDL 10.0	Rep Stress Incr YES Code IRC2006/TPI2002	WB 0.43 (Matrix)	Vert(IL) -0.97 12-13 >496 240 Horz(TL) 0.34 10 n/a n/a	Weight: 176 lb

LUMBER

TOP CHORD 2 X 6 SPF 2100F 1.8E BOT CHORD 2 X 4 SPF 2100F 1.8E WEBS 2 X 4 WW Stud *Except*

5-13,6-13,7-13: 2 X 4 SPF 1650F 1.5E

SLIDER Left 2 X 6 SPF 2100F 1.8E 3-0-4, Right 2 X 6 SPF 2100F 1.8E 3-0-4

BRACING

TOP CHORD BOT CHORD WEBS Structural wood sheathing directly applied or 3-7-13 oc purlins. Rigid ceiling directly applied or 7-6-14 oc bracing.

1 Row at midpt 5-13, 7-13

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=2468/0-5-8, 10=2468/0-5-8

Max Horz 2=85(LC 5)

Max Uplift2=-432(LC 5), 10=-432(LC 6) Max Grav 2=2471(LC 2), 10=2471(LC 3)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/6, 2-3=-6574/1079, 3-15=-6447/108

1-2=0/6, 2-3=-6574/1079, 3-15=-6447/1089, 4-15=-6420/1095, 4-19=-5899/904, 16-19=-5838/908, 5-16=-5816/918,

5-6=-4326/671, 6-7=-4326/671, 7-17=-5816/918, 17-20=-5838/909, 8-20=-5899/904, 8-18=-6420/1096, 9-18=-6447/1090,

9-10=-6574/1080, 10-11=-0/6

BOT CHORD 2-14=-1069/6189, 13-14=-811/5294, 12-13=-726/5294, 10-12=-985/6189

WEBS 4-14=-658/268, 5-14=-36/584, 5-13=-1860/369, 6-13=-198/1692, 7-13=-1860/369, 7-12=-37/584, 8-12=-658/269

NOTES

- Wind: ASCE 7-05; 90mph; TCDL=4.5psf; BCDL=4.5psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) TCLL: ASCE 7-05; Pf=40.0 psf (flat roof snow); Category II; Exp C; Fully Exp.; Ct= 1
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 20.0 psf or 2.00 times flat roof load of 40.0 psf on overhangs non-concurrent with other live loads.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 432 lb uplift at joint 2 and 432 lb uplift at joint 10
- 8) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANS/TPI 1.

LOAD CASE(S) Standard



September 24,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED WITEK REFERENCE PAGE MII-7473 BEFORE USB.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not fixes designer. Bracing shown is for lateral support of Individual web members only. Additional temporary bracing to Insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding flobrication, quality control, storage, delivery, erection and bracing, consult.

AMS/TPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



7777 Greenback Lane, Suite 109 Citrus Heights, CA, 95610 Job Truss Truss Type Qty Ply Jack R29364547 J0809050 T03C GABLE 1 Job Reference (optional) 7.060 s Aug 6 2008 MITek industries, Inc. Wed Sep 24 14:01:43 2008 Page 1 Foxworth Galbraith Truss Company, Delta, CO., Dean McFate 460 6-11-7 13-6-13 26-9-11 33-5-1 40-4-8 40-10-B 0-6-0 6-11-7 6-7-7 8.7.7 8.7.7 6-7-7 6-11-7 0-6-0 Scale = 1:65.3 6x10 = 3.50 12 Ø 5x14 = 13 Ex16 MT18H = 5x16 MT18H = 5x16 MT16H 4x4 () 4x4 || 1-10-0 _1-7-4 11-0-2 20-2-4 29 4 6 9-2-2 9-2-2 0-2-12 9-2-2 9-2-2 0-2-12 1-7-4 Plate Offsets (X,Y): [2:0-2-9,Edge], [2:1-3-14,0-2-0], [2:0-2-13,0-2-8], [4:0-4-0,0-4-8], [5:0-10-0,0-1-15], [7:0-10-0,0-3-0], [8:0-4-0,0-4-8], [10:0-3-0,0-7-8], [10:0-2-13,0-2-8], [10:1-10-3-0,0-1-15], [7:0-10-0,0-3-0], [8:0-4-0,0-4-8], [10:0-3-0,0-7-8], [10:0-2-13,0-2-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3-0,0-7-8], [10:0-3 Edge], [12:0-8-0,0-3-0], [12:0-0-4,0-1-8], [14:0-8-0,0-3-0], [14:0-0-4,0-1-8], [33:0-1-8,0-0-8], [61:0-1-8,0-0-8] LOADING (psf) SPACING CSI DEFL **PLATES** GRIP 2-0-0 in (loc) L/d TCLL 40.0 Plates Increase 1.15 TC 0.46 Vert(LL) -0.51 13-14 >941 360 MT20 169/123 (Roof Snow=40.0) Lumber Increase 1.15 BC 0.90 Vert(TL) -0.97 12-13 >496 240 MT18H 169/123 TCDL 10.0 Rep Stress Incr NO WB 0.43 Horz(TL) 0.34 10 n/a n/a BCLL 0.0 Code IRC2006/TPI2002 Weight: 244 lb (Matrix) BCDL BRACING LUMBER TOP CHORD 2 X 6 SPF 2100F 1.8E BOT CHORD 2 X 4 SPF 2100F 1.8E WEBS 2 X 4 WW Stud *Except* TOP CHORD Structural wood sheathing directly applied or 3-7-9 oc purlins. BOT CHORD Rigid ceiling directly applied or 7-6-9 oc bracing. 5-13, 7-13 1 Row at midpt WEBS 5-13,6-13,7-13: 2 X 4 SPF 1650F 1.5E MiTek recommends that Stabilizers and required cross bracing be

OTHERS 2 X 4 WW Stud

Left 2 X 6 SPF 2100F 1.8E 3-0-4, Right 2 X 6 SPF 2100F 1.8E 3-0-4 SLIDER

REACTIONS (lb/size) 2=2468/0-5-8, 10=2468/0-5-8 Max Horz 2=-85(LC 6)

Max Uplift2=-432(LC 5), 10=-432(LC 6) Max Grav2=2471(LC 2), 10=2471(LC 3)

FORCES (Ib) - Maximum Compression/Maximum Tension

1-2=0/6, 2-3=-6587/1085, 3-67=-6461/1095, 4-67=-6432/1101, 4-71=-5898/902, 68-71=-5836/906, 5-68=-5811/916, TOP CHORD

5-6=-4326/670, 6-7=-4326/670, 7-69=-5811/916, 69-72=-5836/907, 8-72=-5898/902, 8-70=-6432/1102, 9-70=-6461/1096,

9-10=-6587/1086, 10-11=0/6

BOT CHORD 2-14=-1075/6202, 13-14=-813/5298, 12-13=-728/5298, 10-12=-991/6202

4-14=-667/273, 5-14=-32/571, 5-13=-1867/371, 6-13=-200/1699, 7-13=-1867/371, 7-12=-32/571, 8-12=-667/273 WEBS

NOTES

1) Wind: ASCE 7-05; 90mph; TCDL=4.5psf; BCDL=4.5psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.

3) TCLL: ASCE 7-05; Pf=40.0 psf (flat roof snow); Category II; Exp C; Fully Exp.; Ct= 1

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 20.0 psf or 2.00 times flat roof load of 40.0 psf on overhangs non-concurrent with other live loads.

6) All plates are MT20 plates unless otherwise indicated.

7) All plates are 2x4 MT20 unless otherwise indicated.

8) Gable studs spaced at 1-4-0 oc.

9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 432 lb uplift at joint 2 and 432 lb uplift at joint 10.

11) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

ONAL

installed during truss erection, in accordance with Stabilizer

Installation guide

September 24,2008

🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USB. AWARNING: Verify design parameters and read NOTES ON THIS AND TRADUCTURE AT METALOGUE AT IN.

Design valid for use only with Miller connectors. This design is based only upon parameters shown, and is for an individual building component.
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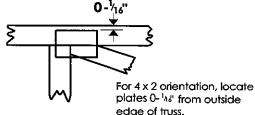
7777 Greenback Lane, Suite 109 Citrus Heights, CA, 95610

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



This symbol indicates the required direction of slots in connector plates.

Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

 4×4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction. Design Standard for Bracina.

DSB-89:

BCSI1:

Building Component Safety Information, Guide to Good Practice for Handling,

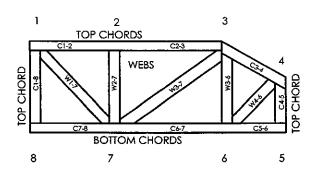
Installing & Bracing of Metal Plate

0.3

Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B, 95-43, 96-31, 9667A NER-487, NER-561 95110, 84-32, 96-67, ER-3907, 9432A

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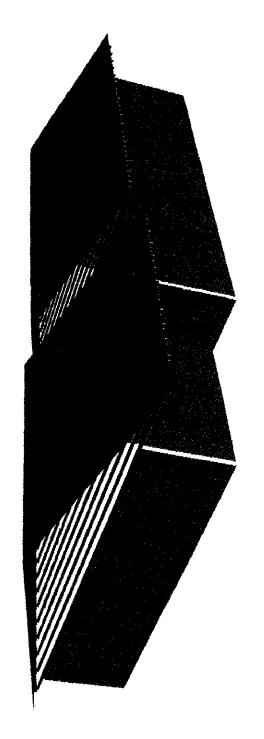
MiTek Engineering Reference Sheet: Mil-7473

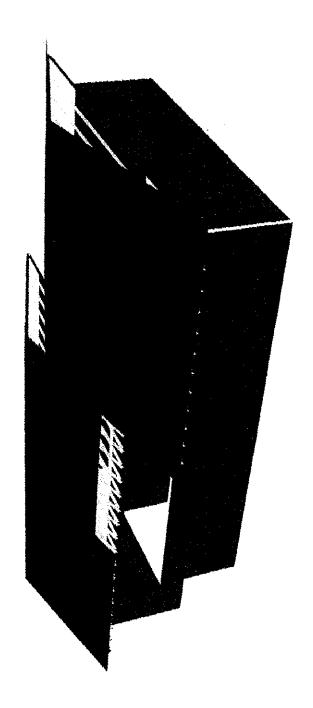
⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

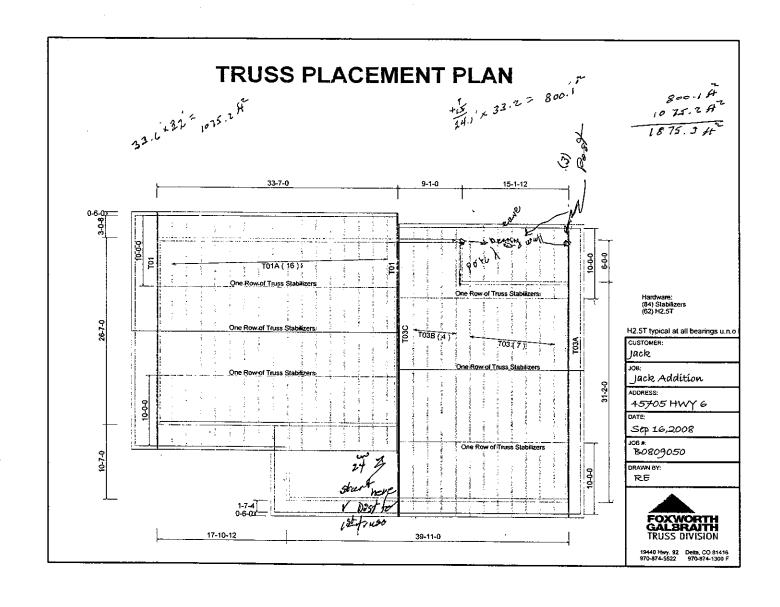
- 1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
- 3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5. Cut members to bear tightly against each other.
- 6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- 7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- 8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- 9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- 10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- 11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- 12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- 13. Top chords must be sheathed or purlins provided at spacing indicated on design.
- 14, Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- 16. Do not cut or after truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- 19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- 20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

Wery 10/16





0809050.BFM 9/16/2008 09:18



						Date	10/24/2008
		BUILD	ING PERMI	T CARD			
Job Address _	45705 Hwy 6	8&24, GWS / Cany	on Creek Est	ates, Lot	t 69		
Owner	Panter, Rich	ard	Address_	45705	Hwy 6&24, GWS	Phone	# 970-947-5203
Contractor	Foxworth G	albraith	Address_	19440	Hwy 92, Delta	Phone	# 970-874-5522
Setbacks: F	ront	Rear	RH		_ LH	Zonir	ng
re-roof -	This is a	new -	INSPECTION	IS_			
Soils Test			1	Neathern	oroofing		
Footing							
Foundation				Electrica	Rough (State)		
Grout				Electrica	I Final (State)_		
Underground F	Plumbing						leted?
Rough Plumbi	ng						
Framing 3	9-10-9m2						
Insulation	The second secon		(Septic S			
Roofing							
	establish Santania						
			(Other	-		
			NOTES				

11155

(continue on back)

2123-361-22-001

Assessor's Parcel No. ____

FINAL CHECKLIST

EXTERIOR	MECHANICAL ROOM	CTA IOWA VO	
Address No.	Boiler	STAIRWAYS	
Drainage		, ,	
Decks-support & clearance to wood			
Decks-stairs & rails	Floor drain	· · · · · · · · · · · · · · · · · · ·	
Exterior locks	Clearance		
Flashing around doors & windows			
insect screens	Hot water heater		
	Combustion air		
INTERIOR	Gas piping, valves		
GARAGE	LPG Drain		
Fire wall separation	FIREPLACE/STOVE	•	
Service doors-1%" min		Smoke detectorBATHROOMS	
Door (20 min.) w/auto closer-1%" min.			
Mech. equip. 18" above floor			
No opening into sleeping area			
BASEMENT-CRAWL AREA	Glass doors		
Access	Certified by:		

REMARKS

COUNTY OF GARFIELD - BUILDING DEPARTMENT

CORRECTION NOTICE

108 8th St., Suite 401 Glenwood Springs, Colorado Phone (970) 945-8212

Job located at 45705 7404 6+24

Permit No. ///55

	cted this structure and these e following corrections needed:
Nadd Bracing Per.	truss drawings
Vollent Venting	to outside
3) Veriby Bearing	of true with
many Jan T	have a steer real
Letter shaving	locations of Bleine
1) access to other	Com statte
5) ak to shingle 11	ith letter from trus
Communes	y state of
11-17-08 Will nee	d letter formorgineer
for Expesting stru	ctem
Call for Re-Inspection	
□ \$50.00 Re-Inspection Fee mu	st be paid prior to Re-Inspection
	he above correction must be inspected ore covering.
When correction(s) have been	made, call for inspection at 970-384-5003.
Date //-17-08	20
Building Inspector mutto	mound
Phone (970) 945-8212	7702000
(0.0) 0.0 02.2	

LDING PERM

GARFIELD COUNTY, COLORADO

THIS CARD IS POSTED ON THE JOB

Date Issued 10-24-08

11155

AGREEMENT

In consideration of the issuance of the permit, the applicant hereby agrees to comply with all laws and regulations related to the zoning, location; construction and erection of the proposed structured for which this permit is granted, and further agrees that if the above said regulations are not fully complied with in the zoning, location, erection and construction of the above described structure, the permit may then be revoked by notice from the County Building department and IMMEDIATELY BECOME NULL AND VOID.

Address or Legal Description 45705 Hwy let 24 GWS

Owner Panter, Pichard Contractor Foxworth Galbraith

Building Permit Type Pesidential

This Card Must Be Posted So It Is Plainly Visible From The Street Until Final Inspection

INSPECTION RECORD

Footing	Driveway
Foundation / Grouting	Insulation
Underground Plumbing	Drywall
Rough Plumbing	Electric Final (by State Inspector) (Prior to Final)
Rough Mechanical	Septic Final
Gas Piping	FINAL 6-25-12 7M
Electric Rough (by State Inspector)	(You Must Call For Final Inspection)
(Prior to Framing)	Notes
Framing 3-9-10 OK 9m2 (odd nan)	
(to include Roof in place & Windows & Doors installed & Firestopping in place)	

THIS PERMIT IS NOT TRANSFERABLE

For Inspection Call 970-384-5003 Office 970-945-8212 108 8th Street, Suite 401 Glenwood Springs, Colorado 81601

DO NOT DESTROY THIS CARD

PROTECT PERMIT FROM WEATHER DAMA

(DO NOT LAMINATE)